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A1 BARS-

B1 BARS-

B2 BARS

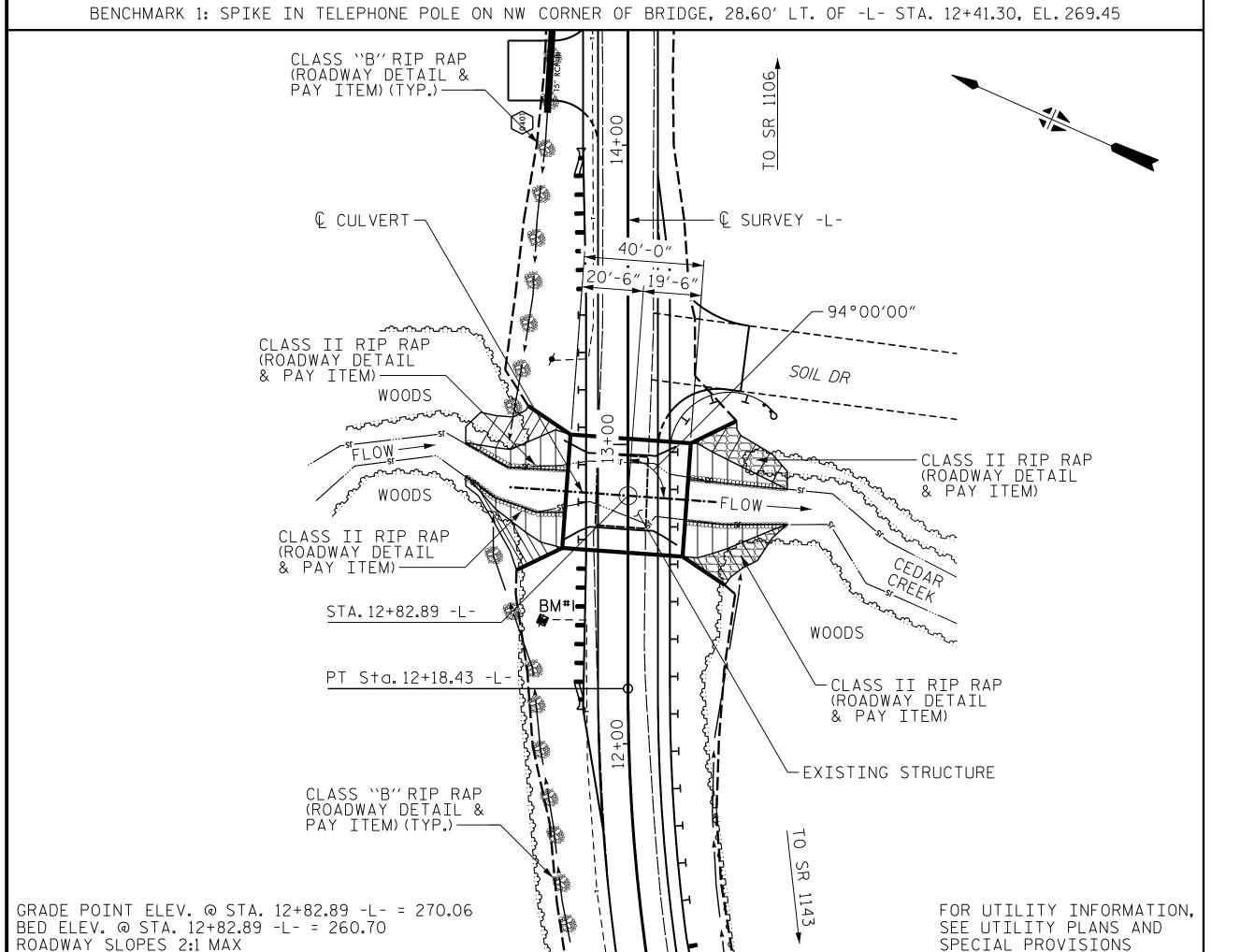
A2 BARS—

\_ DATE : <u>9/11/14</u>

\_ DATE : <u>9/19/14</u>

T. ANDREWS

P.ERVIN



LOCATION SKETCH

5-C1 BARS @ 3'-O" CTS. MAX. (TYP. EXT. FACES)

(BARRELS 1 AND 3)

12'-0"

2" HIGH BEAM BOLSTERS (B.B.) @ 4'-0" CTS.

-A100 BARS

\* ALL CONTINUOUS HIGH CHAIR

 $-*3\frac{1}{2}$ " HIGH C.H.C.U.

A400 BARS-

DWG BY:

CHK BY:

UPPER (C.H.C.U.)@ 3'-0" CTS.

12-C1 BARS @ 1'-O" CTS. (TYP. INT. FACES)

(ALL BARRELS)

W. TOWE

-\* 10<sup>1</sup>/4" HIGH C.H.C.U.

 $-3'' \varnothing WEEP HOLES (TYP.)$ 

38′-8″

-A300 BARS

SEE DETAIL "A'

PERMITTED CONST. JT.-

RIGHT ANGLE SECTION OF BARREL

(LOOKING DOWNSTREAM)
THERE ARE 140 "C" BARS IN SECTION OF BARREL

\_\_\_ DATE : <u>8/29/14</u>

T. ANDREWS DATE: 9/26/14

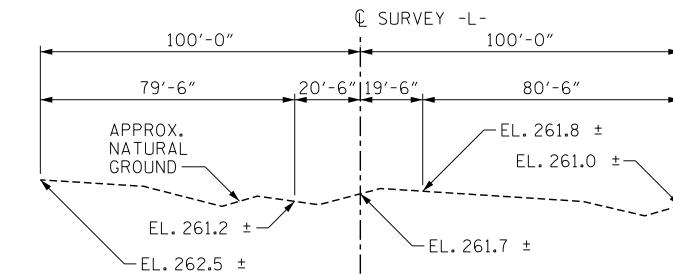
6'-0"

—— PERMITTED CONST. JT.

C1 BARS @ 4 EQ. SPA.

4" (TYP.)

(BARREL 2)



# PROFILE ALONG & CULVERT

# HYDRAULIC DATA

DESIGN DISCHARGE =	1100 CFS
FREQUENCY OF DESIGN FLOOD =	25 YR.
DESIGN HIGH WATER ELEVATION =	269.4
DRAINAGE AREA=	3.12 SQ.MI.
BASIC DISCHARGE =	1500 CFS
BASIC HIGH WATER ELEVATION =	270.14

# OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	=	1100 CF
FREQUENCY OF OVERTOPPING FLOOD	=	25 YR.
OVERTOPPING FLOOD ELEVATION	=	269.7

TOTAL STRUCTURE QUA	ITITNA	ES	
CLASS A CONCRETE			
BARREL @3.807 C.Y./FT	152.3		C.Y.
WINGS, SILLS, ETC.	42.8		C.Y.
TOTAL	195.1		C.Y.
REINFORCING STEEL			
BARRELS, SILLS & HEADWALLS	22,316		LBS.
WINGS	2,338		LBS.
TOTAL	24,654		LBS.
CULVERT EXCAVATION		LUMP	SUM
PLACEMENT OF NATIVE MATERIAL		LUMP	SUM
REMOVAL OF EXSITING STRUCTURE		LUMP	SUM
FOUNDATION CONDITIONING MATERIAL		120 7	ONS
			•

├── (CULVERT (BARRELS SYMM. ABOUT THIS LINE)

OVERTOPPING DISCHARGE	=	1100 CFS
FREQUENCY OF OVERTOPPING FLOOD	=	25 YR.
OVERTOPPING FLOOD ELEVATION	=	269.7

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REMOVAL OF EXSITING STRUCTURE		LUMP SUM
FOUNDATION CONDITIONING MATERIAL		120 TONS

#### NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

DESIGN FILL = 2.67 FT.

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTES SHEET.

3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN THE CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

- 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON THE WING SHEETS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN (1 @ 25'-6") WITH A TIMBER DECK ON I-BEAMS AND A CLEAR ROADWAY OF 17'-1" ON TIMBER CAPS, POSTS AND SILLS (WITH A HELPER BENT AT END BENT 2) AND LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 12+82.89.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

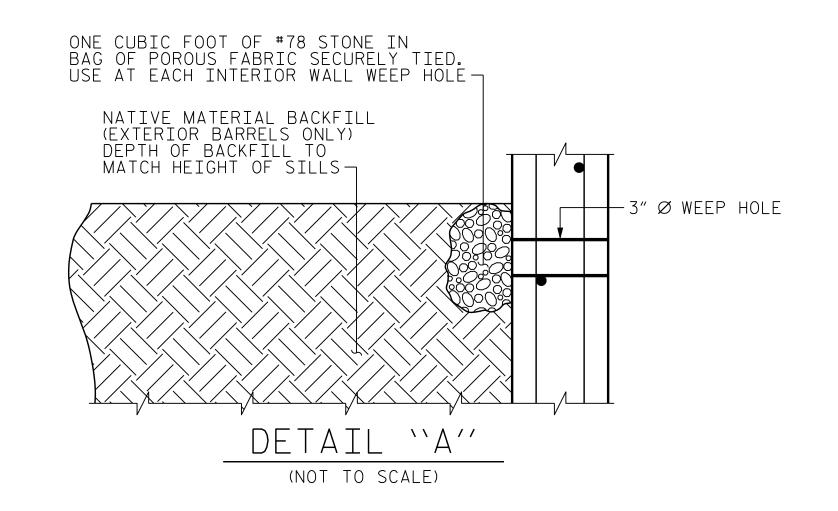
STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES WILL BE PAID FOR BY THE CONTRACTOR.

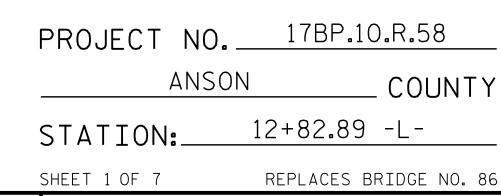
A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

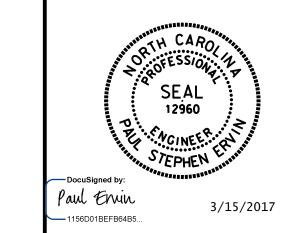
THE REQUIRED BEARING CAPACITY AT THE BASE OF THE CULVERT IS 1 TSF. THE REQUIRED BEARING CAPACITY SHALL BE VERIFIED.

THE REINFORCED CONCRETE BOX CULVERT SHALL BE PLACED ON THE STANDARD 1.0 FOOT BLANKET OF FOUNDATION CONDITIONING MATERIAL.

- FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.
- NATIVE MATERIAL SHALL BE USED TO BACKFILL THE CULVERT BETWEEN THE SILLS. SEE SPECIAL PROVISIONS FOR "PLACEMENT OF NATIVE MATERIAL".







STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

TRIPLE BARREL 12 FT. X 7 FT. CONCRETE BOX CULVERT

> 94° SKEW REVISIONS NO. BY: DATE: S-01

SHEET NO. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 TOTAL SHEETS



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LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
STRENGTH I LIMIT STATE																
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W x RF	LIVE-LOAD FACTORS (Y <sub>ll</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.11		1.75	1.18	1	BOTTOM SLAB	12.67	1.11	1	TOP SLAB	11.65	-
DESIGN		HL-93 (OPERATING)	N/A		1.44		1.35	1.53	1	BOTTOM SLAB	12.67	1.44	1	TOP SLAB	11.65	-
LOAD RATING	NG	HS-20 (INVENTORY)	36.000	2	1.17	42.1	1.75	1.18	1	BOTTOM SLAB	12.67	1.17	1	TOP SLAB	11.65	-
		HS-20 (OPERATING)	36.000		1.52	54.7	1.35	1.53	1	BOTTOM SLAB	12.67	1.52	1	TOP SLAB	11.65	_
		SNSH	13.500		2.45	33.0	1.40	2.45	1	TOP SLAB	5.40	3.47	1	TOP SLAB	11.65	-
		SNGARBS2	20.000		2.29	45.8	1.40	2.29	1	TOP SLAB	5.40	2.80	1	BOTTOM SLAB	12.07	-
	ICLE	SNAGRIS2	22.000		2.36	51.9	1.40	2.36	1	BOTTOM SLAB	12.67	2.54	1	BOTTOM SLAB	12.07	-
	VEHICLI (V)	SNCOTTS3	27.250		1.48	40.3	1.40	1.57	1	TOP SLAB	5.40	1.48	1	TOP SLAB	11.65	-
	$\prod_{S} \prod_{S} \prod_{i \in S} \prod_{s \in S} \prod_{i \in S} \prod_{s \in S} \prod_{i \in S} \prod_{s \in S} \prod_$	SNAGGRS4	34.925		1.49	52.0	1.40	1.49	1	BOTTOM SLAB	12.67	1.61	1	BOTTOM SLAB	12.07	-
	INGL	SNS5A	35.550		1.46	51.9	1.40	1.46	1	BOTTOM SLAB	12.67	1.58	1	BOTTOM SLAB	12.07	-
	S	SNS6A	39.950		1.32	52.7	1.40	1.32	1	BOTTOM SLAB	12.67	1.41	1	BOTTOM SLAB	12.07	-
LEGAL		SNS7B	42.000		1.27	53.3	1.40	1.27	1	BOTTOM SLAB	12.67	1.35	1	BOTTOM SLAB	12.07	_
LOAD RATING	ER	TNAGRIT3	33.000		1.61	53.1	1.40	1.61	1	BOTTOM SLAB	12.67	1.70	1	BOTTOM SLAB	12.07	-
	RAIL	TNT4A	33.075		1.59	52.5	1.40	1.59	1	BOTTOM SLAB	12.67	1.70	1	BOTTOM SLAB	12.07	-
	/I-TR	TNT6A	41.600		1.27	52.8	1.40	1.27	1	BOTTOM SLAB	12.67	1.35	1	BOTTOM SLAB	12.07	-
	SEMI- ST)	TNT7A	42.000		1.26	52.9	1.40	1.26	1	BOTTOM SLAB	12.67	1.34	1	BOTTOM SLAB	12.07	-
	TOR (TT	TNT7B	42.000		1.30	54.6	1.40	1.30	1	BOTTOM SLAB	12.67	1.34	1	BOTTOM SLAB	12.07	-
	TRAC	TNAGRIT4	43.000		1.25	53.7	1.40	1.25	1	BOTTOM SLAB	12.67	1.31	1	BOTTOM SLAB	12.07	-
	TRUCK	TNAGT5A	45.000		1.19	53.5	1.40	1.19	1	BOTTOM SLAB	12.67	1.25	1	BOTTOM SLAB	12.07	-
	TRL	TNAGT5B	45.000	(3)	1.18	53.1	1.40	1.18	1	BOTTOM SLAB	12.67	1.25	1	BOTTOM SLAB	12.07	-

# 12'-0" (TYP.) BOX 2 вох з

# LRFR SUMMARY (LOOKING DOWNSTREAM)

 DES BY:
 T. ANDREWS
 DATE:
 9/11/14
 DWG BY:
 W. TOWE
 DATE:
 8/29/14

 DES CHK:
 P. ERVIN
 DATE:
 9/19/14
 CHK BY:
 T. ANDREWS
 DATE:
 9/26/14

LOAD FACTORS:

#### DESIGN LOAD RATING FACTORS

BESTON EOAD	1170	1 701011.
LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	-
WA	1.00	_

# NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

# COMMENTS:

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING \*\*
- \*\* SEE CHART FOR VEHICLE TYPE

PROJECT NO. 17BP.10.R.58 ANSON \_\_\_\_ COUNTY STATION: 12+82.89 -L-



SHEET 2 OF 7

3/8/2017

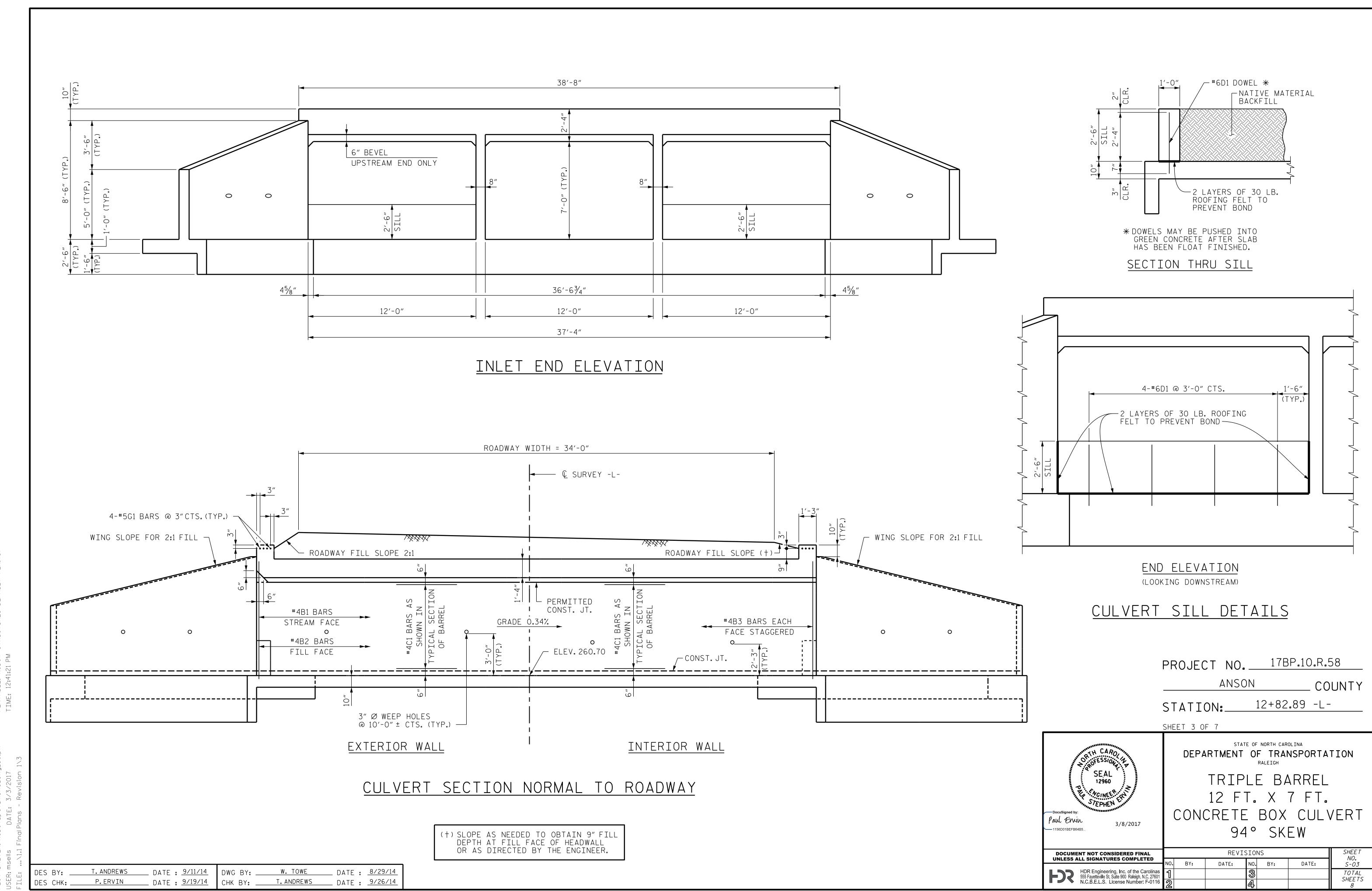
STATE OF NORTH CAROLINA

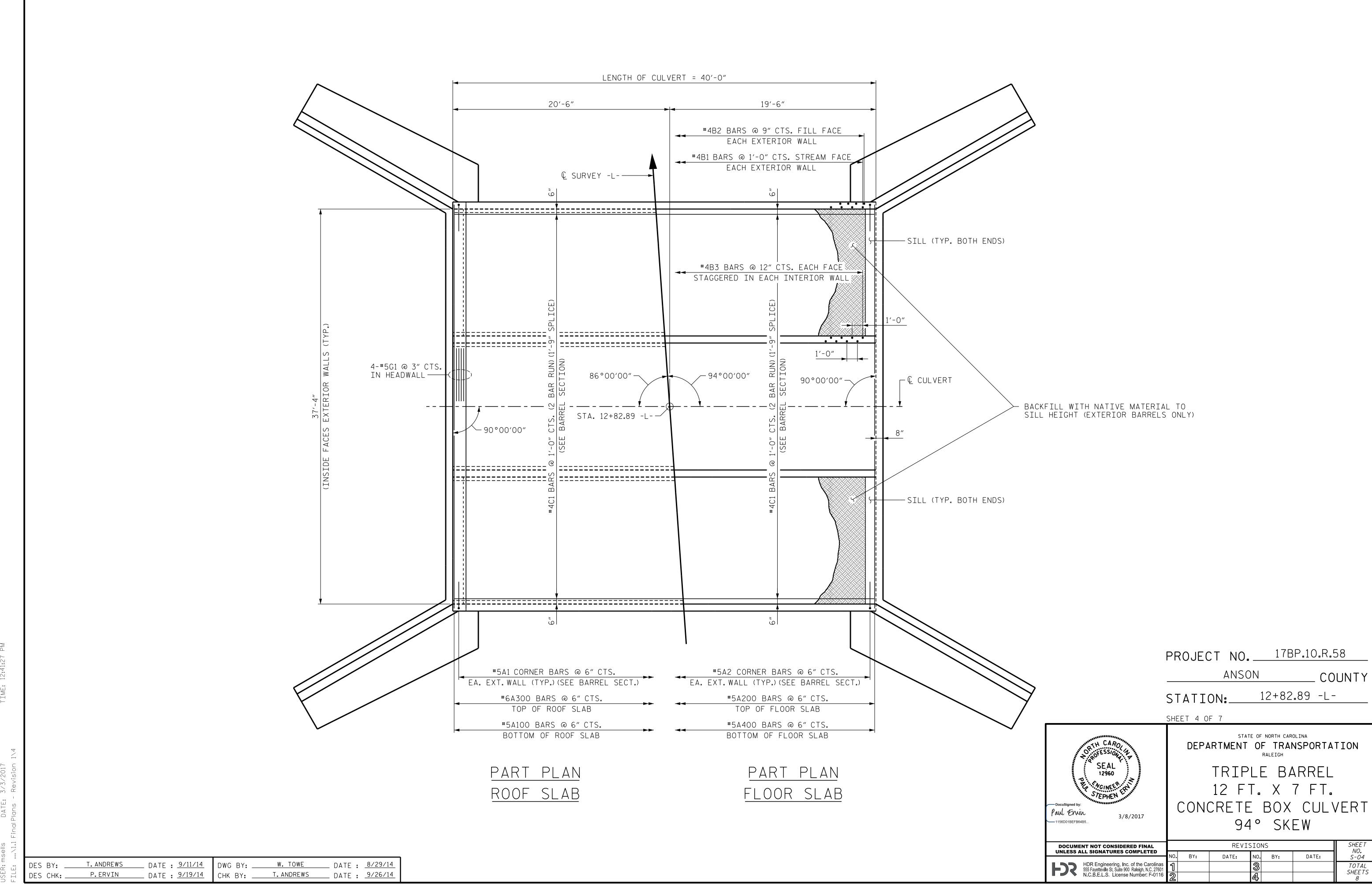
DEPARTMENT OF TRANSPORTATION
RALEIGH

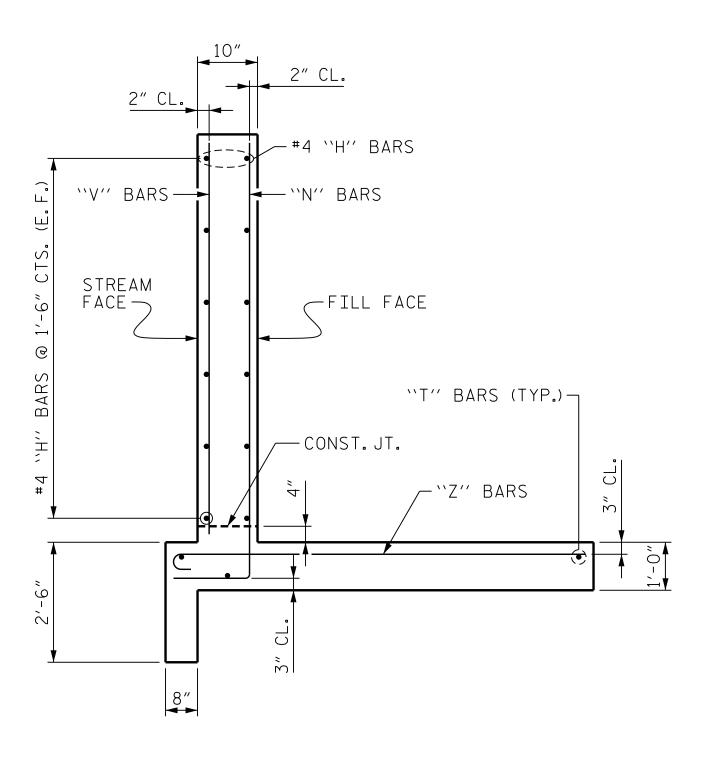
TRIPLE BARREL 12 FT. X 7 FT. CONCRETE BOX CULVERT 94° SKEW

TOTAL SHEETS 8

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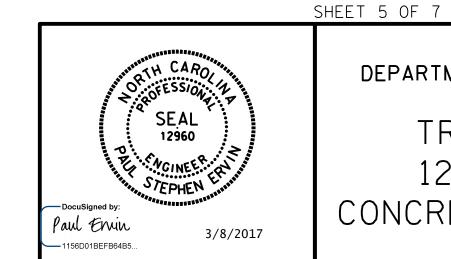


# TYPICAL WING SECTION

# NOTES

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

PROJECT NO. 17BP.10.R.58 ANSON COUNTY STATION: 12+82.89 -L-



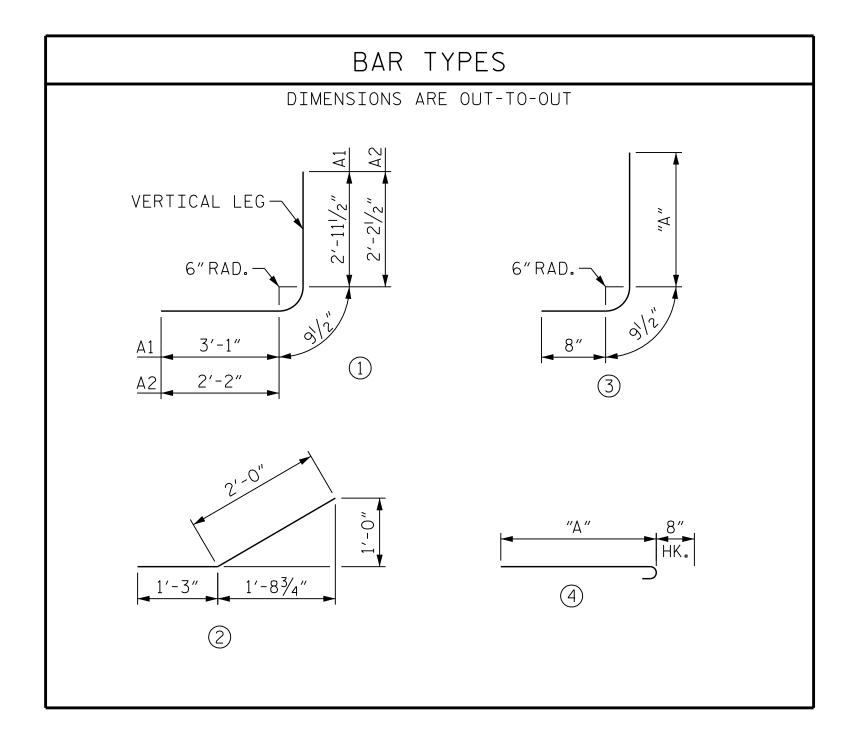
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

TRIPLE BARREL 12 FT. X 7 FT. CONCRETE BOX CULVERT 94° SKEW

SHEET NO. S-05 REVISIONS DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED NO. BY: DATE: HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 TOTAL SHEETS 8

CULVERT BARREL										
REINFORCING STEEL BAR SCHEDULE										
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
A1	160	#5	1	6′-10″	1,140					
A2	160	#5	1	5′-2″	862					
	•									
A100	80	#5	STR.	38'-4"	3,199					
A200	80	#5	STR.	38′-4″	3,199					
A300	80	#6	STR.	38′-4″	4,606					
A400	80	#5	STR.	38′-4″	3,199					
B1	80	#4	STR.	8'-9"	468					
B2	106	#4	STR.	6′-2″	437					
В3	160	#4	STR.	8′-9″	935					
C1	280	#4	STR.	20′-9″	3,881					
D1	16	#6	STR.	2'-11"	70					
G1	8	#5	STR.	38'-4"	320					
REINFORCI	NG STEEL			LBS.	22,316					

		CULVE	RT WING	G WALL		
	REINF	ORCING	STEEL	BAR SCH	HEDULE	
BAR	NO.	SIZE	TYPE	LENGTH	DIM. A	WEIGHT
H1	24	#4	STR.	15′-2″		243
H2	8	#4	STR.	14'-4"		77
Н3	8	#4	STR.	7′-1″		38
Н4	8	#4	STR.	15′-5″		82
H5	48	#4	2	3'-3"		104
N1	8	#5	3	9'-9"	8'-31/2"	81
N2	8	#5	3	9'-5"	7'-11 /2"	79
N3	8	#5	3	9'-0"	7'-61/2"	75
N4	8	#5	3	8'-7"	7'-11/2"	72
N5	8	#5	3	8'-2"	6'-81/2"	68
N6	8	#5	3	7'-9"	6'-31/2"	65
N7	8	#5	3	7'-4"	5'-101/2"	61
N8	8	#5	3	6'-11"	5'-51/2"	58
N9	8	#5	3	6'-7"	5'-11/2"	55
113	<u> </u>		<u> </u>		J 1/2	] 33
S1	12	#6	STR.	6'-0"		108
		_	- <del></del> -	T .= .		T
T1	12	#5	STR.	17'-0"		213
V1	8	#4	STR.	7′-8″		41
V2	8	#4	STR.	7′-5″		40
V3	8	#4	STR.	7'-0"		37
V4	8	#4	STR.	6′-7″		35
V5	8	#4	STR.	6′-2″		33
V6	8	#4	STR.	5′-9″		31
V 7	8	#4	STR.	5′-4″		29
V8	8	#4	STR.	4'-11"		26
V9	8	#4	STR.	4′-6″		24
7.1		H.C.		T 5/ 0"	T	
Z1	8	#6	4	5′-9″	5'-1"	69
Z2	16	#6	4	5′-6″	4'-10"	132
Z3	16	#6	4	5′-3″	4'-7"	126
Z4	16	#6	4	5′-0″	4'-4" 4'-2"	120
Z5	16	#6	4	4'-10"	4'-2"	116
REINFORCI FOR 4 WIN						2,338 LBS
4 CUL					<b>-</b>	30.0 CY 3.6 CY 4.4 CY 4.8 CY
					TOTA	L: 42.8 CY



_			
	SPLICE	LEN	GTH CHART
	BAR	SIZE	SPLICE LENGTH
	C1	4	1'-9"
	A100	5	2'-2"
	A200	5	2'-2"
	A300	6	3′-10″
	A400	5	2'-2"

PROJECT NO. 17BP.10.R.58 ANSON \_\_\_\_ COUNTY STATION: 12+82.89 -L-

SHEET 6 OF 7

Paul Ervin 1156D01BEFB64B5...

3/8/2017

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

TRIPLE BARREL 12 FT. X 7 FT. CONCRETE BOX CULVERT 94° SKEW

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	SHEE 7 NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	5-06
1			3			TOTAL SHEET
2			AL			SHEET

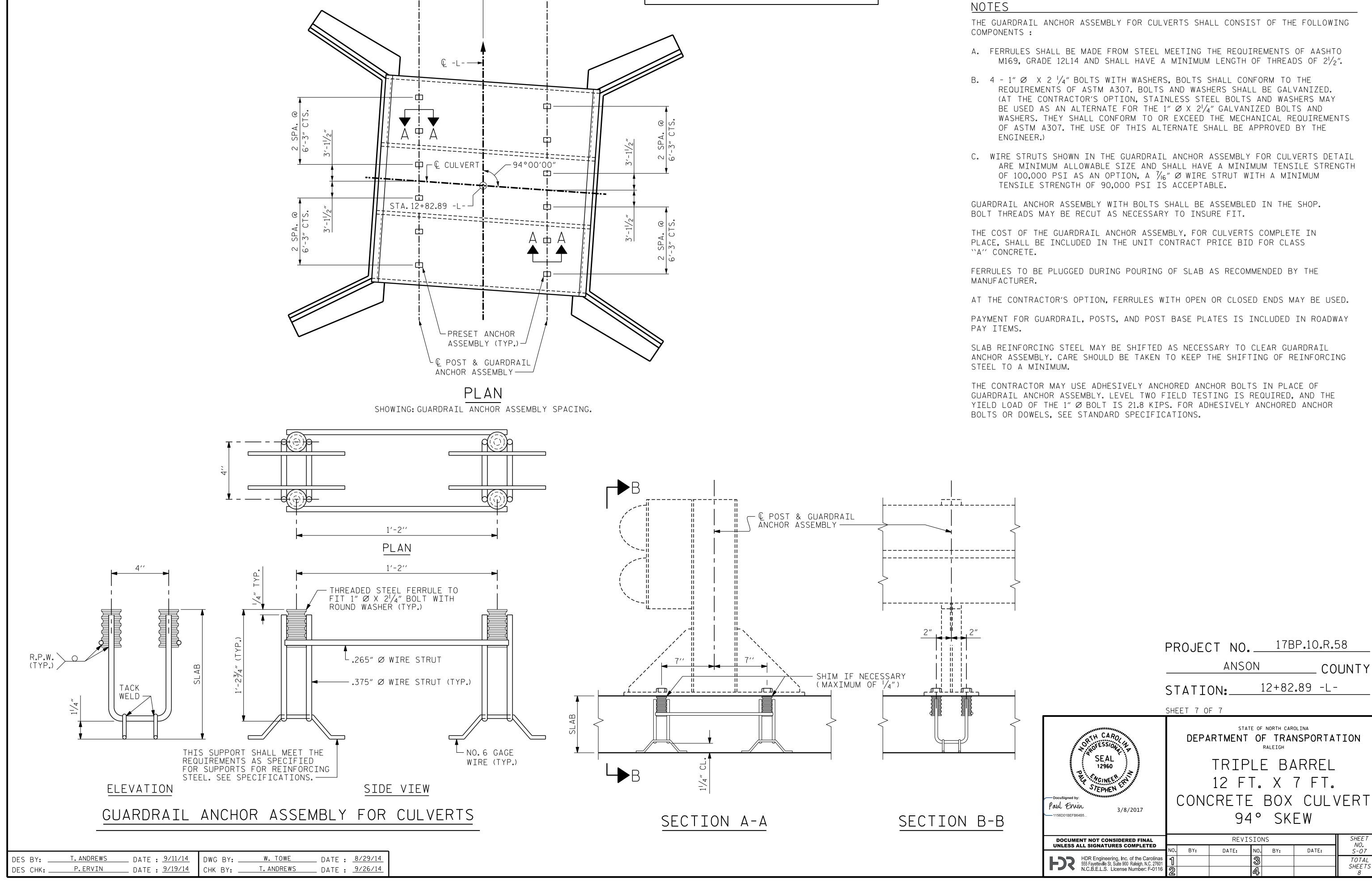
 

 DES BY:
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 P. ERVIN
 DATE: 9/19/14

 DWG BY:
 W. TOWE
 DATE:
 8/29/14

 CHK BY:
 T. ANDREWS
 DATE:
 9/26/14



(中) DIMENSION TO BE FURNISHED BY THE ENGINEER

15'-2" ± (中) , 15'-2" ± (中)

# STANDARD NOTES

#### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	HL 93
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50W -	27,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS. PER CU. FT.

# MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO  $\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A  $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A  $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

# REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/6" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY  $\frac{1}{16}$  OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

PROJECT NO. 17BP.10.R.58

ANSON COUNTY

STATION: 12+82.89 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

BALETCH

STANDARD NOTES

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-08

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REVISIONS

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TOTAL SHEETS
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